## **APPENDIX F-C**

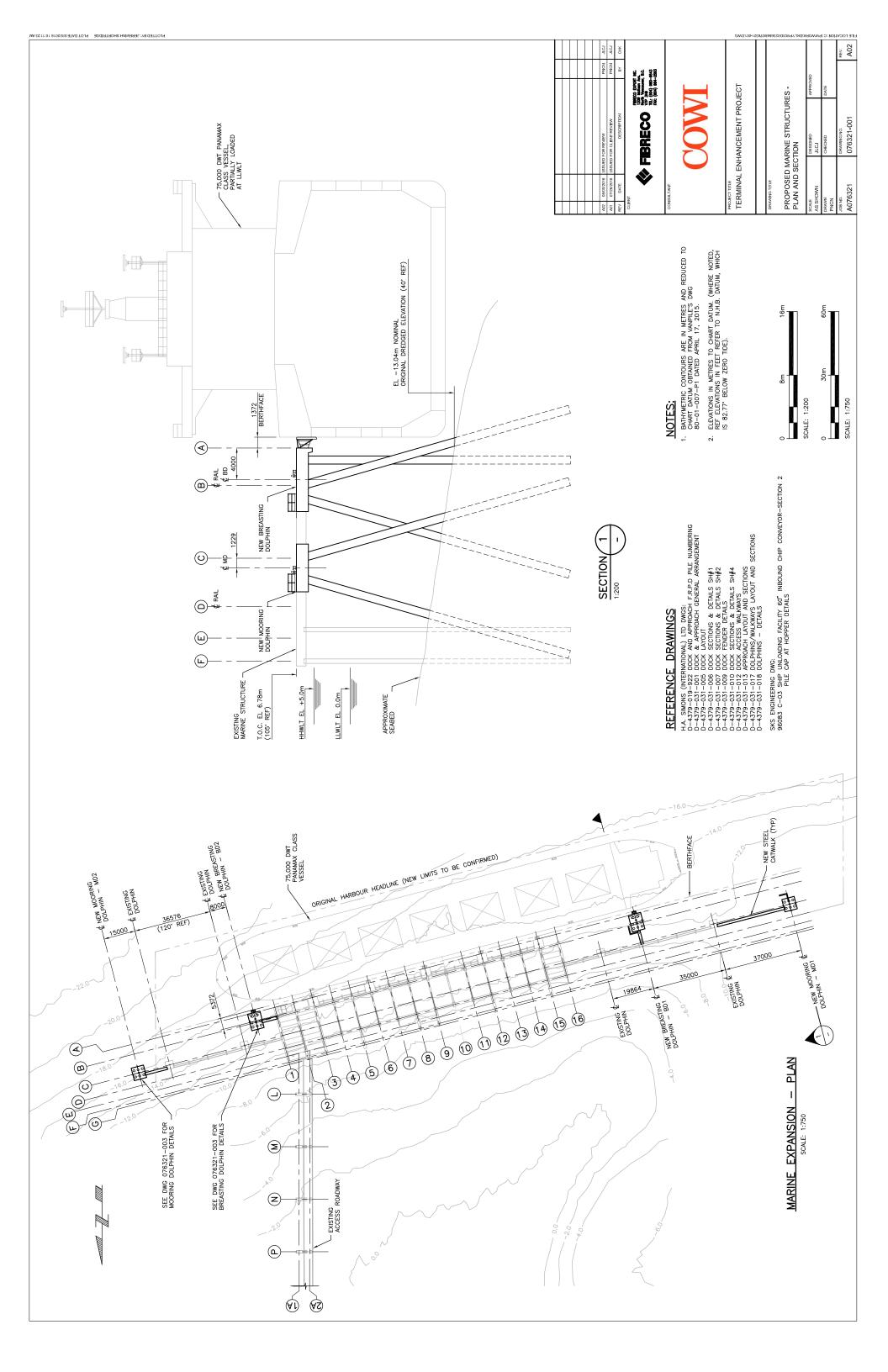
## **Marine Structure Drawings (R2)**

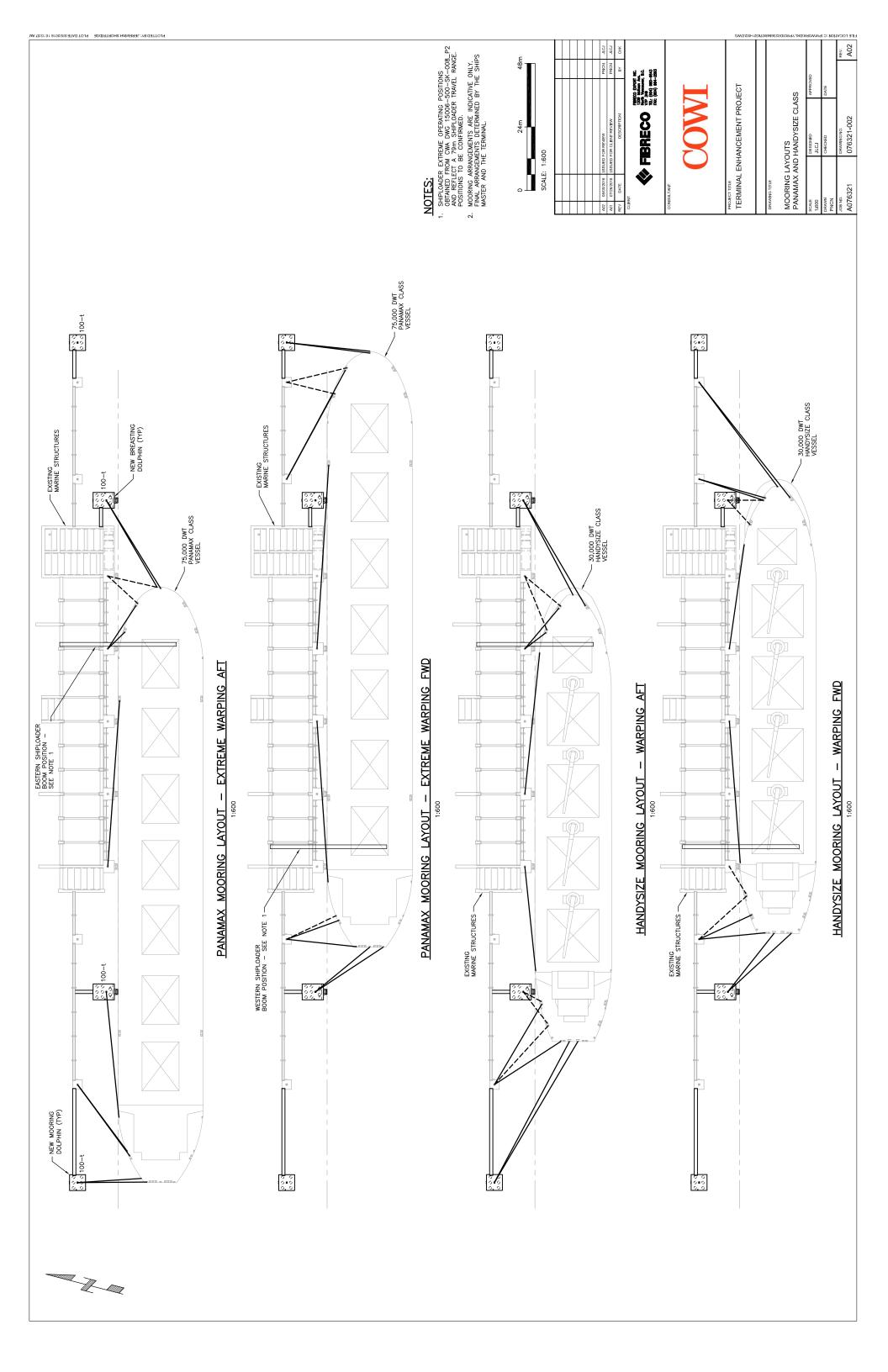


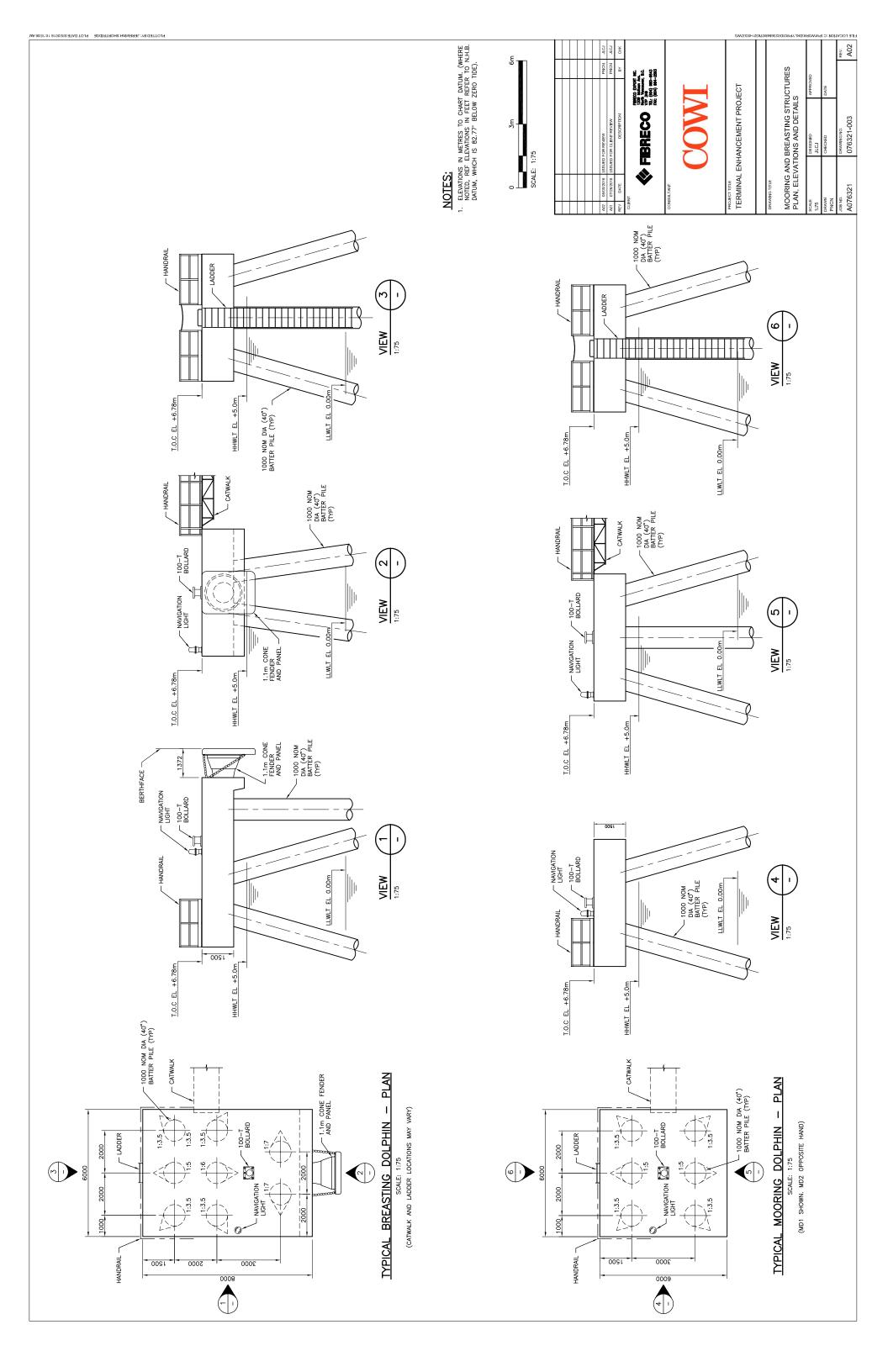


For Vancouver Fraser Port Authority Review Only

Fibreco Export Terminal Enhancement Permit Application Document







Evergreen Engineering PO Box 21530 Eugene, OR 97402-0409 Ph: (541) 484-4771 Fax: (541) 484-6759 BRUKS Fibreco Loader Preliminary Loads Eugene S. Cochrane Job #3219.1 3/9/2016

## Fibreco Ship Loader Loading -

The structure will be located at the Fibreco site in VAncouver BC

The platform is a Risk Category II structure - In order to maintain the use of service level wind speeds, the structure will be designed with ASCE 7-05, Allowable Stress Design.

Wind Loading - ASCE 7-05; Sec 6.5.15 Design Wind Load on Other Structures	1.03
Basic wind speed (3sec gust at 33ft) v := 35 Design wind speed in operation	1.08
m · · · · · · · · · · · · · · · · · · ·	1.12
Use exposure D  K <sub>xt</sub> := 1.0 no topographic effects -	1.16
	1.22
€:= 1.09 gust effect factor - ASCE 7-05 6.5.8.2	1.27
K <sub>d</sub> := .85 wind directionality factor - ASCE 7-05 Table 6-4	1.31
volocity proceure coefficient ASCE 7.05 and Table 6.2	1.34
velocity pressure coefficient - ASCE 7-05 sec. Table 6-3 $K_z$ :=	1.38
$q_z := .00256psf \cdot K_z \cdot K_{zt} \cdot K_d \cdot v^2$ velocity pressure - ASCE 7-10 sec. 6.5.10	1.40
$\varepsilon_{\rm h} = .1$ $C'_{\rm f} = 4 \cdot \varepsilon^2 - 5.9 \cdot \varepsilon + 4 = 3.45$ Note that Cf is intended to represent the wind on the entire	1.43
tower applied to a single face, for RISA, divide Cf by 2 and	1.48
apply wind to both faces equally.	1.52
$C_f := \frac{C_f}{2} = 1.725$ Use this value for loader gantry. Boom will be treated as a solid - cladded building.	1.55
	1.58
$L_m := 192ft$ B := 12ft For wind normal to narrow face $\kappa_n := \frac{L}{B}$	1.61
$C_{fn} := if \left(\kappa_n < 1.5, \frac{1}{4} \cdot \kappa_n^2 + 1.4, \frac{2}{3} \cdot \kappa_n + .9\right) = 11.567$ normal to wide face $\kappa_w := \frac{B}{L}$	

 $C_{fw} := if \left( \kappa_w < 1.5, \frac{1}{4} \cdot \kappa_w^2 + 1.4, \frac{2}{3} \cdot \kappa_w + .9 \right) = 1.401$ 

Wind on narrow face -

Wind on gantry -

$$F_n := q_z \cdot G \cdot C_f = \begin{cases} 5.413 \\ 5.613 \\ 5.814 \\ 6.115 \\ 6.365 \\ 6.566 \\ 6.716 \\ 6.917 \\ 7.017 \\ 7.167 \\ 7.167 \\ 7.418 \\ 7.618 \\ 7.769 \\ 7.919 \\ 8.069 \end{cases}$$

5.162

Wind on Side face of Boom/Counter Boom -

MId height of boom is 80 feet, therefore use the kz at 80 feet

$$F_{fw} := q_{z_{\underline{o}}} C_{fw} \cdot \textbf{\textit{G}} = 5.617 \cdot psf$$

Use a 10'-6" boom height and get load at top on bottom chords -

$$w_{fw} := \frac{F_{fw} \cdot 10.5ft}{4} = 14.746 \cdot plf$$
 wind line load on each chord

Wind on End of Boom -

$$\textbf{F}_{fn} \coloneqq \textbf{q}_{\textbf{z}_{g}} \cdot \textbf{\textit{C}}_{fn} \cdot \textbf{\textit{G}} = \textbf{46.378} \cdot \textbf{psf}$$

Distribute end of boom loads along the length of the top and bottom chords -

$$w_{fn} := \frac{F_{fn} \cdot 10.5ft \cdot 20ft}{4 \cdot 177.5ft} = 13.717 \cdot plf$$

For wind on the boom in the Up position -

$$L_u := 15ft$$
  $B_u := 10.5ft$  For wind normal to narrow face  $\kappa_{nu} := \frac{L_u}{B_u}$ 

$$C_{fnu} := if \left( \kappa_{nu} < 1.5 \,, \frac{1}{4} \cdot \kappa_{nu}^{2} + 1.4 \,, \frac{2}{3} \cdot \kappa_{nu} + .9 \right) = 1.91 \qquad \qquad \text{normal to wide face} \qquad \kappa_{wu} := \frac{B_{u}}{L_{u}}$$

$$\text{Cfw} = if \left( \kappa_w < 1.5, \frac{1}{4} \cdot \kappa_w^2 + 1.4, \frac{2}{3} \cdot \kappa_w + .9 \right) = 1.401$$

$$q_{\overline{W}} = .00256psf \cdot K_z \cdot K_{zt} \cdot K_d \cdot v^2$$

· psf

0 100.412

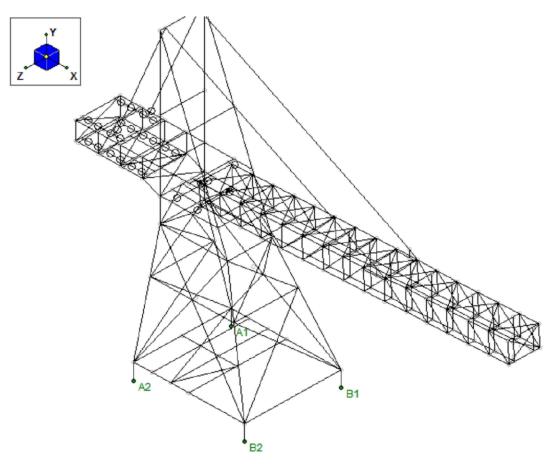
105.287 109.186

	3	113.086
	4	118.935
	5	123.81
10.5	6	127.709
$w_{nu} := q_z \cdot G \cdot C_{fnu} \cdot \frac{1000}{4} = 0$	7	130.634
•	8	134.533
	9	136.483
	10	139.408
	11	144.282
	12	148.181
	13	151.106
	14	154.031

15

156.955

(	Operating Loads - Shuttle Back										
		Dead	Material	Snow	Plugged Chute	Wind X	Wind Z	Seis X	Seis Y	Seis Z	
,	A1	136.993	-6.441	14.304	-14.029	-10.185	-45.242	-41.597	17.809	-124.145	
	Α2	123.954	-5.817	14.227	-14.057	-10.204	45.244	-41.694	16.114	124.156	
	B1	125.644	14.796	108.245	31.529	10.185	10.678	41.597	16.334	19.788	
	B2	125.903	14.887	108.285	31.557	10.204	-10.68	41.694	16.367	-19.799	
Total Vert.	Load	512.494	17.425	245.061	35				66.624		
Total Horiz	. React	ion				-15.9	-22.2	-77		-77	
	Operat	ing Loads -	Shuttle O	ut							
		Dead	Material	Snow	Plugged Chute	Wind X	Wind Z	Seis X	Seis Y	Seis Z	
,	A1	97.269	-7.836	-2.633	-45,401	-8.752	-40.893	-35.238	12.645	-124.958	
,	A2	128.981	-1.117	29.385	-25.116	-11.666	40.983	-48.481	16.767	125.211	
	B1	165.246	20.843	125.178	62.901	8.752	6.327	35.238	21.482	20.214	
I	B2	121.96	14.124	93.13	42.616	11.666	-6.416	48.481	15.855	-20.467	
Total Vert.	Load	513.456	26.014	245.06	35				66.749		
Total Horiz	. React	ion				-15.9	-22.2	-77		-77	
(	Operat	ing Loads -	Boom Up							Storm	Wind
		Dead	Snow	Wind X	Wind Z	Seis X	Seis Y	Seis Z		X	Z
,	A1	183.951	58.798	-37.493	-61.172	-48.651	23.616	-99.71		-280.77	-449.153
,	A2	187.551	70.96	-38.641	61.236	-57.114	24.052	99.662		-289.367	449.78
	B1	78.479	63.751	37.493	-9.41	48.651	10.069	-29.855		280.77	-79.396
	B2	63.327	51.551	38.641	9.345	57.114	8.124	29.903		289.367	78.769
Total Vert.	Load	513.308	245.06				65.861				
Total Horiz	. React	ion		-33.3	-29.5	-75		-76		-249.3	-221



Estimated Vertical Loads	for Fibreco Si	hip Loader.	
Loads given are PRELIMIN	NARY and not	for design pu	rposes
Vertical loads are positiv	e down		
Design Conditions:			
2000 US TPH (at belt spee	ed equal to 11	ft/s)	
Operating wind speed - 3	SMPH		
Max. wind speed (parked	and tied dov	vn) - 95MPH	
Wind loads are given for	ASCE 7-05 Exp	oosure catego	ry 'D'
Snow Load (60psf)			
Seismic Coefficient - Cs=	0.15		
All loads are SERVICE levi	el loads		
Wind and seismic loads a	re reversible		

UNLESS NOTED OTHERWISE DIMENSIONS ARE IN INCHES
DO NOT SCALE DRAWING
DO NOT SCALL DIVAVING

WEIGHT (ONE ASSEMBLY)

UNLESS NOTED OTHERWISE THIRD ANGLE PROJECTION

FRAC +/- 1/16 .XX +/- .020 .XXX +/- .005 ANGLES +/- 1 DEG

TOLERANCES

PROPRIETARY AND CONFIDENTIA

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	P1	2/1916	GSC	REVISED PROPOSAL
	Р	1/29/16	KH	PROPOSAL
	REV.	DATE	BY	DESCRIPTION

RAIL LOADS SHIP LOADER

	SCALE	DATE 1/29/16	
	DRAWN BY	APPROVED BY	
	DESIGNED BY KH	CHECKED BY	
	REPLACES	SHEET 1 of 1	
SHEET SIZE	PROJECT 14-005234		
В	DRAWING NO.		P1
_	. •		

